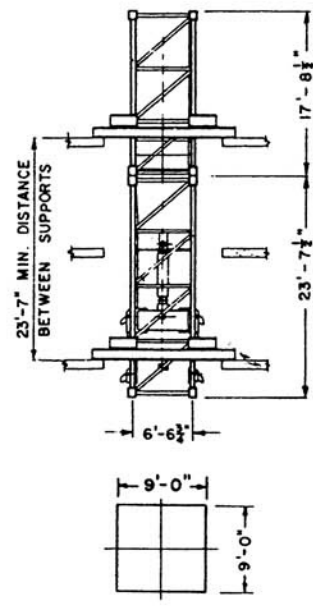
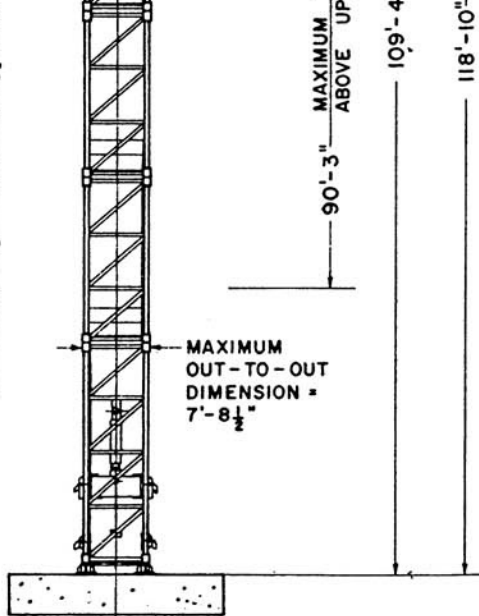


LOADING DIAGRAM

WHEN WORKING IN THIS LOADRANGE, OPERATION IS RESTRICTED TO ONE MOTION AT A TIME (EITHER HOISTING, SLEWING OR TROLLEY MOVEMENT) WITH MAXIMUM WINDSPEED RESTRICTED TO 20 M.P.H.  
SPECIAL LOADING APPLIES ONLY FOR 128'-0" JIB.



SLAB OPENING CONFIGURATION B/I



FOUNDATION PAD CONFIGURATION A/I

NOTES:

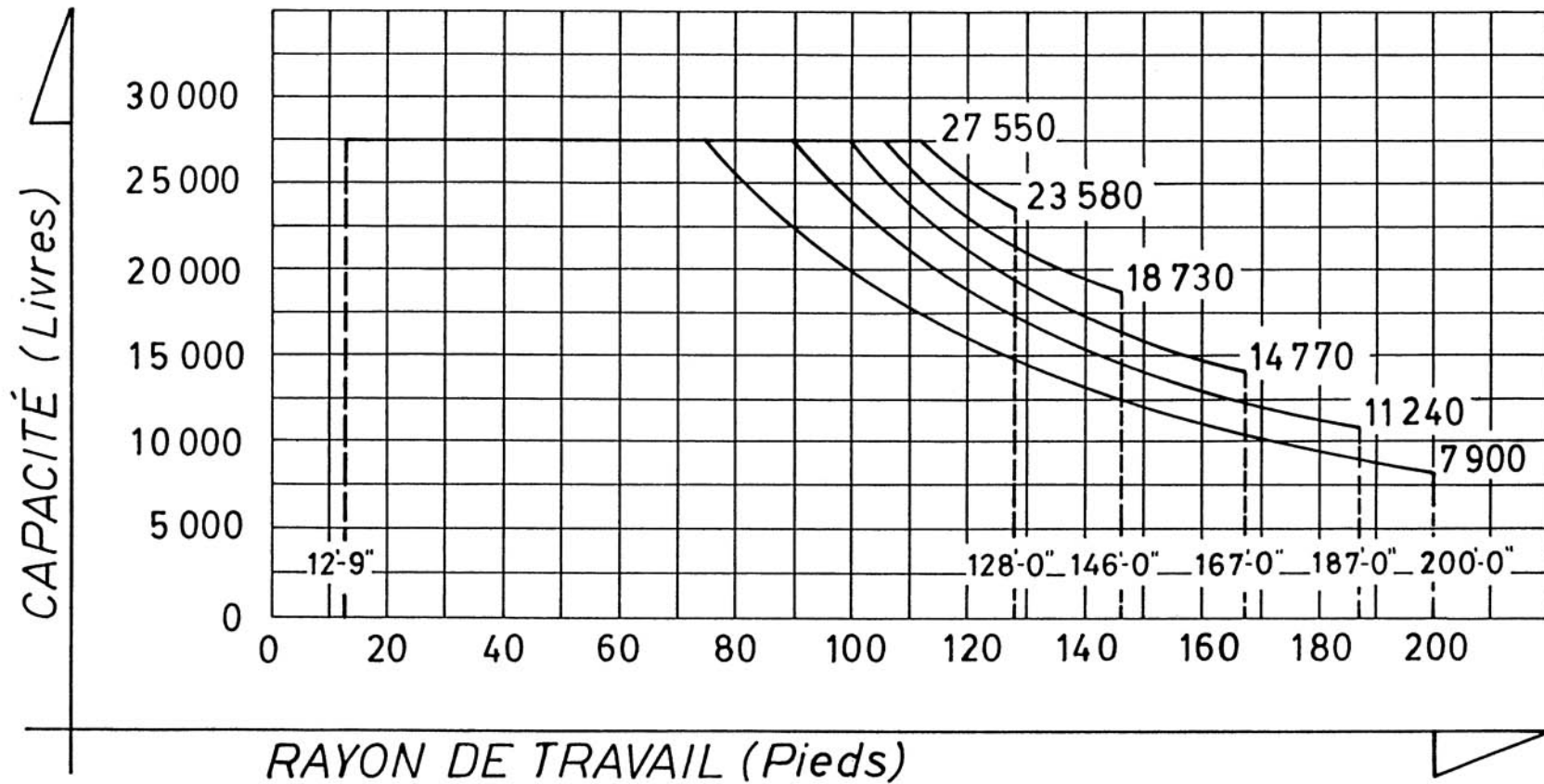
- FOR FOUNDATION & SLAB OPENING DETAILS SEE DRAWINGS PC 4400-9 & -10 AND RESPECTIVELY PC 4400-9S & -10.
- FOR MACHINERY SEE TECHNICAL DATA SHEETS FOR PC 4400.
- SPECIFICATIONS ARE SUBJECT TO CHANGE WITHOUT NOTICE.
- MINIMUM SLAB OPENING 8'-8 3/8" x 8'-8 3/8" WITH CORNER PLATES.



**Armand Guay Inc.**  
Division des grues à tour  
1-514-354-4420

CLIMBING CRANE MODEL  
PC 4400 S

# DIAGRAMME DES CHARGES



**GUAY**

*DIVISION des Grues a Tour*  
1-514-354-4420

PC 4400 S  
2 BRINS  
GROUPE I

**NOTE:**

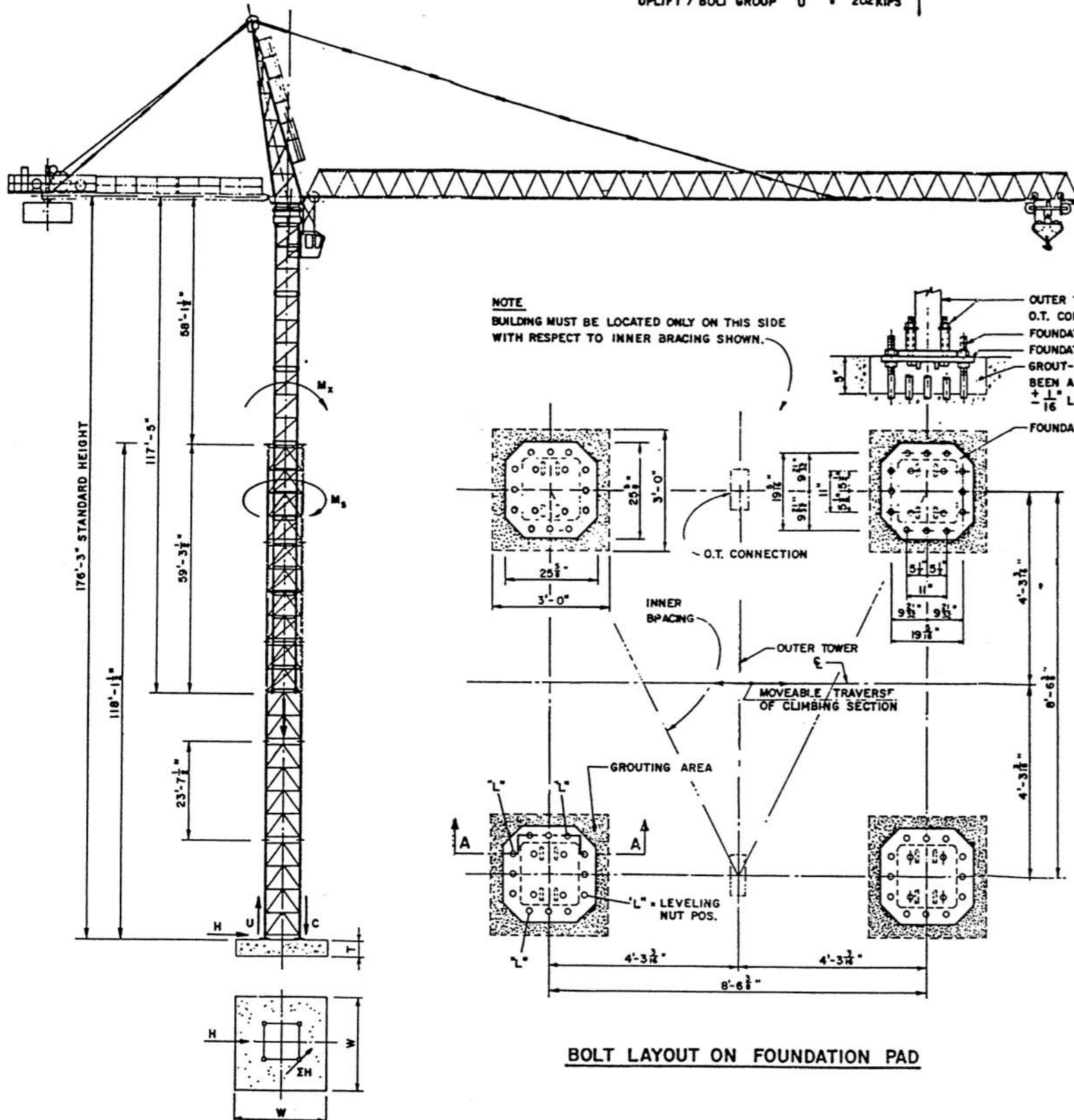
WORKING CONDITIONS ARE RESTRICTED TO WINDSPEEDS UP TO 45 MILES PER HOUR. IF WINDSPEED EXCEEDS 45 MPH THE CRANE MUST BE FREE TO SWING LIKE A WEATHERVANE.

**REACTION FORCES**

OVERTURN MOMENT	$M_o = 3,551$ FT.KIPS	SLEWING MOMENT	$M_s = 456$ FT.KIPS
VERTICAL LOAD (MIN)	$V_{min} = 257$ KIPS	SHEAR / BOLT GROUP	$\Sigma H = 22.5$ KIPS
VERTICAL LOAD (MAX)	$V_{max} = 365$ KIPS	HORIZONTAL SHEAR <sup>in service</sup>	$H_1 =$
COMPRESSION / CORNER	$C = 387$ KIPS	HORIZONTAL SHEAR <sup>out of service</sup>	$H_o =$
UPLIFT / BOLT GROUP	$U = 202$ KIPS		

**36 KIPS MAX.**

TABLE B		MATERIAL LIST		
ITEM #	QTY	DESCRIPTION	MATERIAL	REMARKS
1	4	1" $\square$ 26" x 26"	ASTM A36	(12) $\frac{1}{4}$ " $\phi$ Holes Required to match foundation plate holes.
2	48	$\frac{1}{2}$ " $\phi$ Threaded Rod	"	Length = Dimension "T" (Table C)
3	160	$\frac{1}{2}$ " $\phi$ Heavy Hex. Nuts	ASTM A325	$\frac{1}{2}$ " $\phi$ Washer Req'd For E. Nut.

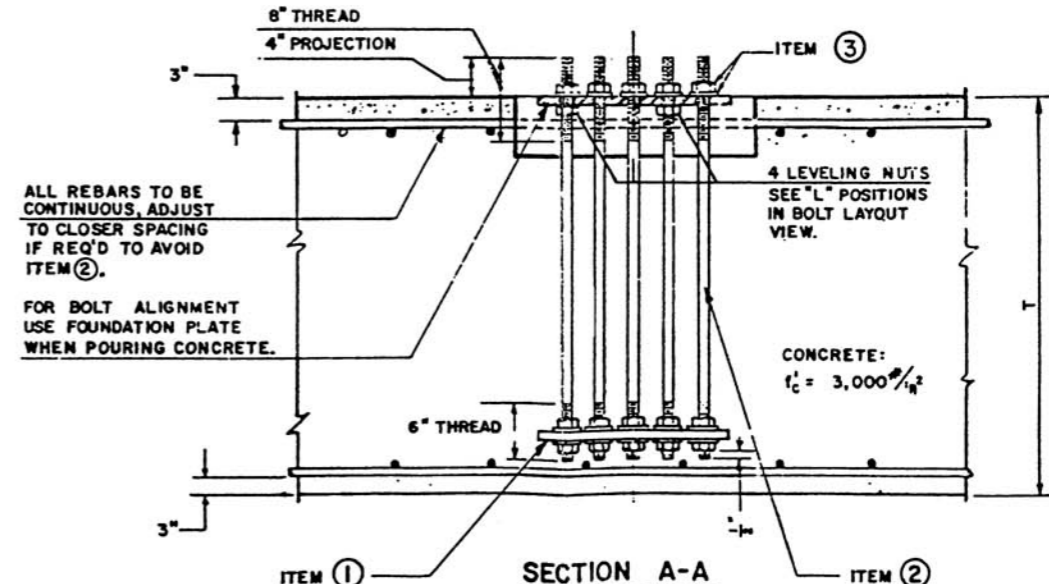


**NOTE:**  
BUILDING MUST BE LOCATED ONLY ON THIS SIDE WITH RESPECT TO INNER BRACING SHOWN.

OUTER TOWER LEG ASSEMBLED WITH O.T. CONNECTION BOLTS TENSIONED.  
FOUNDATION BOLT - ITEM ②  
FOUNDATION PLATE  
GROUT-IN AFTER LOWER TOWER HAS BEEN ADJUSTED TO LEVEL WITHIN  $\pm \frac{1}{16}$ " LEG TO LEG, USING NON-SHRINKING GROUT.

TABLE C		FOUNDATION SPECIFICATION:			
SOIL PRESSURE	W	T	REINFORCING AT TOP*	REINFORCING AT BOTTOM*	WEIGHT OF FOUNDATION
1 TON/ft. <sup>2</sup>	26'-6"	3'-6"	#4 @ 24"	#10 @ 12"	369 KIPS
1.5 TON/ft. <sup>2</sup>	23'-0"	4'-0"	#5 @ 12"	#9 @ 12"	317 KIPS
2.0 TON/ft. <sup>2</sup>	21'-6"	4'-5"	#5 @ 12"	#9 @ 12"	312 KIPS

CALCULATIONS ARE BASED ON CONCRETE WITH A SPECIFIC WEIGHT OF 150  $\frac{\text{lb}}{\text{ft.}^3}$ .  
\* USE ASTM A615-40 REINFORCING BARS IN BOTH DIRECTIONS.  
 $f'_c = 20,000 \frac{\text{psi}}{\text{in.}^2}$



**NOTE:**  
FOUNDATION DETAILS AND REACTION FORCES SHOWN APPLY ONLY TO THE CRANE CONFIGURATION SHOWN IN ELEVATION. SEE APPROPRIATE CONFIGURATION SHEETS FOR OTHER APPLICATIONS.

**GENERAL NOTES**  
1. This drawing is provided by AMERICAN PECCO CORPORATION as a service without liability on its part and is to be approved by your engineer. American Pecco Corporation is not to be held responsible for any deviation from this drawing.  
2. Contractor to check and verify all dimensions on site before proceeding with work.  
3. Member dimensions on this drawing are the final dimensions over scales dimensions.  
4. Contractor is responsible for rebar.

**ELEVATION & FOUNDATION PLAN**

**BOLT LAYOUT ON FOUNDATION PAD**

REVISIONS

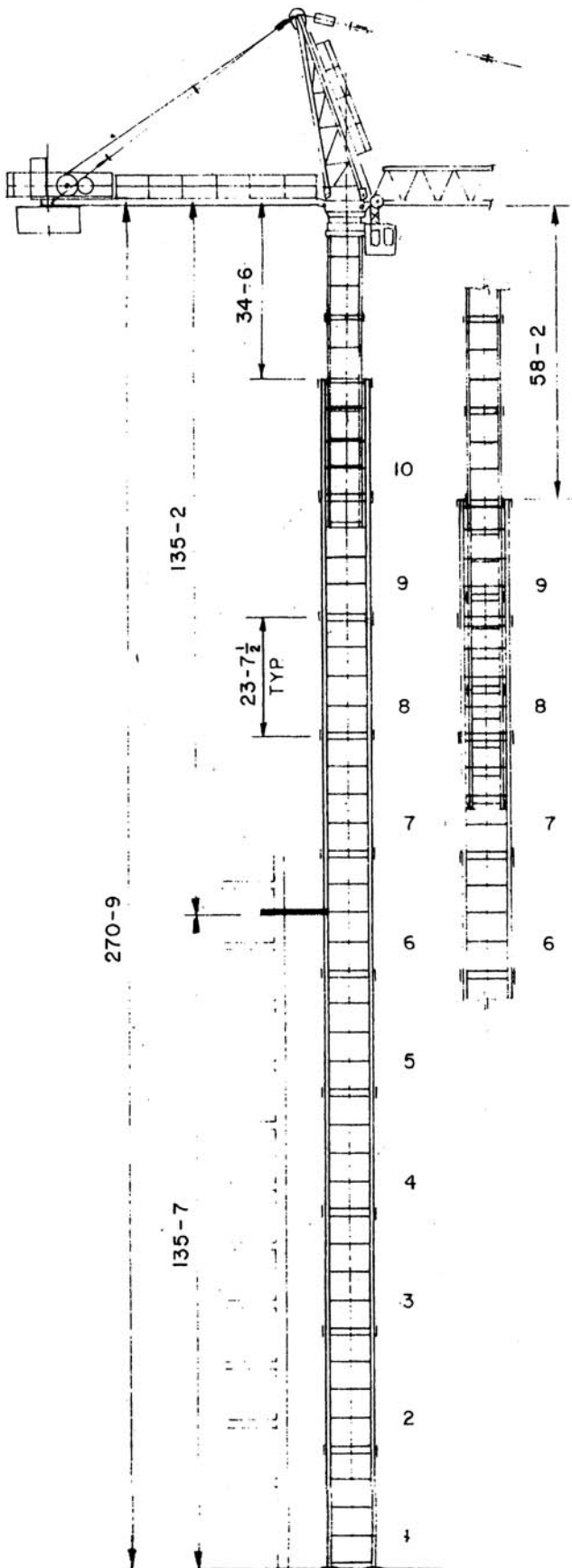
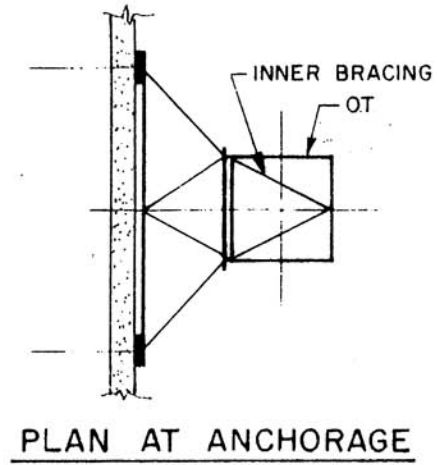


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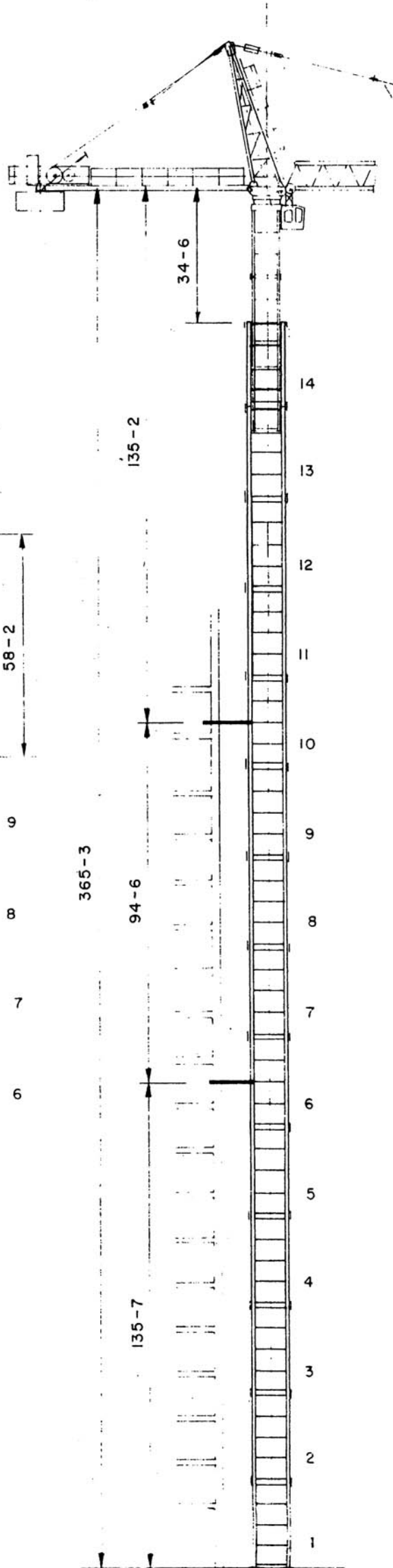
SUBJECT:		<b>FOUNDATION DETAILS &amp; REACTION FORCES</b>	
		<b>PC 4400IN OUTER TOWERS</b>	
DISTRIBUTOR:		<b>(3600 O.T.)</b>	
DRAWN BY:	G.K.W.	CHECKED BY:	
SCALE:	NOT TO SCALE	DATE:	NOV. 1975
DWG. NO.:	OT/PC 4400		

**"D" CONFIGURATIONS**  
**OT 3600 + PC 3600**  
**OT 4400 + PC 4400**

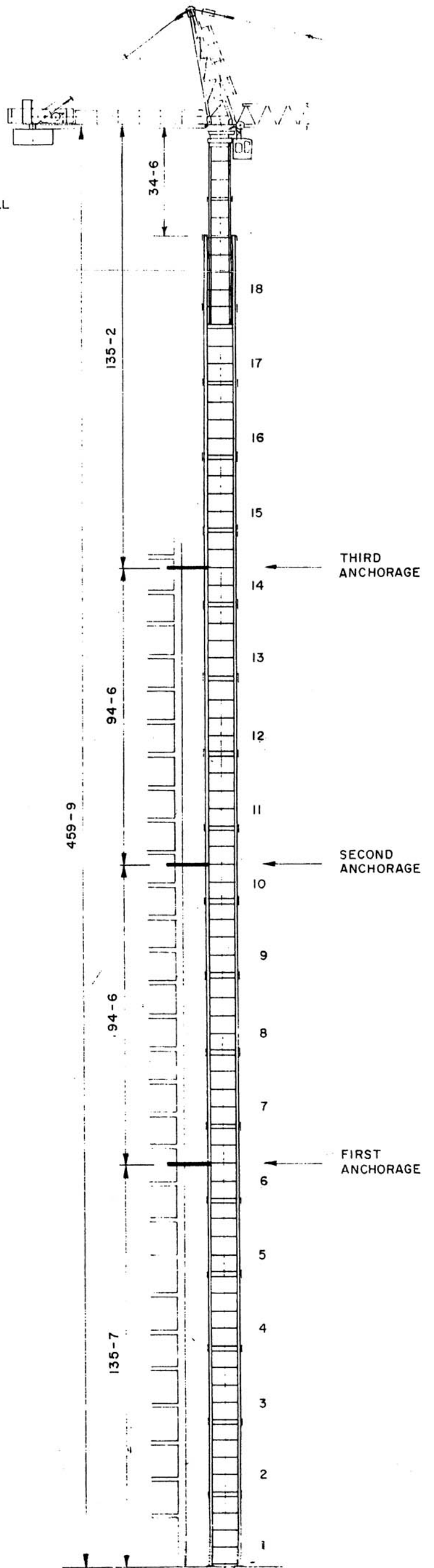
**NOTE:** FOR CONFIGURATIONS SHOWN, USE ONE LESS O.T. SECTION WHEN FULL INNER TOWER CRANE IS EMPLOYED.



"D/1"



"D/2"



"D/3"